S T E E L B R I 💭 G E S

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11th INTERNATIONAL SYMPOSIUM ON STEEL BRIDGES 2024

Investigations On the Effective Width of Wide Flange Steel Girders

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Presenting: Vasil Nikolov

Prague 2024.09

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Effective width of wide flange steel girder according to EN



κ	Verification	β – value		
$\kappa \leq 0,02$		$\beta = 1,0$		
$0,02 < \kappa \le 0,70$	sagging bending	$\beta = \beta_1 = \frac{1}{1+6,4\kappa^2}$		
	hogging bending	$\beta = \beta_2 = \frac{1}{1 + 6.0 \left(\kappa - \frac{1}{2500 \kappa}\right) + 1.6 \kappa^2}$		
> 0,70	sagging bending	$\beta = \beta_1 = \frac{1}{5.9 \kappa}$		
	hogging bending	$\beta = \beta_2 = \frac{1}{8,6 \kappa}$		
all ĸ	end support	$\beta_0 = (0,55 + 0,025 / \kappa) \beta_1$, but $\beta_0 < \beta_1$		
all ĸ	Cantilever	antilever $\beta = \beta_2$ at support and at the end		
$\kappa = \alpha_0 b_0 / L_e$ w in which A_{sl} is	with $\alpha_0 = \sqrt{1 + \frac{A_{s\ell}}{b_0 t}}$ the area of all longitude defined in Figure 3.1 and	dinal stiffeners within the width b_0 and other		

Table 3.1: Effective^s width factor β

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Analytical models for shear lag evaluation



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Recalculated parameters of shear lag



struct. part	k	0.1	22	2	12
	case	β	<i>a</i> ₁	<i>a</i> ₂	<i>a</i> ₃
deck plate	a	0.9442	5.9097	0.2108	2.022
	b	0.8684	10.523	0.2033	2.669
	<i>c</i> ₁	0.9107	9.8026	0.2060	2.587
0	<i>C</i> ₂	0.9108	9.7975	2.061	2.586
cantilever	a	0.9382	6.5852	0.2167	1.708
plate	b	0.9343	7.0332	-0.019	
	С	0.9454	5.7738	0.0321	
bottom plate	a	0.9485	5.4300	0.2100	2.021
	b	0.8552	16.930	0.1895	3.428
	<i>c</i> ₁	0.8722	14.647	0.1924	3.361
EC3		0.9398	6.4	0.2	4.0

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"Au" approach in deck plate between girders



Effective breadth ratio β_i .

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"Au" approach in deck plate between girders





c)

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"Au" approach in cantilever deck parts



stresses for a cantilever deck plate

b) at the deck plate

stress diagrams along the transverse elements c) at bottom plate d) at deck plate

Distribution of transversal stresses along web axis of girder due to bending



Diagram of transverse direct stresses along the longitudinal edge of model 2

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"Au" approach for unequally loaded girders



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Conclusion



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Thank you for your attention

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